



GOVERNING EQUATIONS AND COMPLETE MATHEMATICAL PROOFS FOR COULOMB'S THEORY OF EARTH PRESSURE

Tse-Shan Hsu

President, Institute of Mitigation for Earthquake Shear Banding Disasters
Professor, Department of Civil Engineering, Feng-Chia University, Taiwan, R.O.C.
tshsu@fcu.edu.tw

Yi-Ju Chen

Ph.D. Student, Ph.D. Program of Mechanical and Aeronautical Engineering,
Feng-Chia University, Taiwan, R.O.C.

Zong-Lin Wu Tzu-Che Chao Ding-Yu Wei

Directors, Institute of Mitigation for Earthquake Shear Banding Disasters
Taiwan, R.O.C.

Jiann-Cherng Yang Yi-Min Huang

Associate and Assistant Professors, Feng-Chia University, Respectively,
Taiwan, R.O.C.

Abstract

Coulomb's theory of earth pressure, proposed in 1776, has been widely used by engineers to design retaining walls for 245 years. However, because the earth pressure functions are very complicated, there is still no study providing governing equations and complete mathematical proofs. Therefore, this paper provides governing equations for solving Coulomb's theory of earth pressure using the first-order differential equations of Coulomb's earth pressure functions. Furthermore, the second-order differential equations of Coulomb's earth pressure functions are used to mathematically prove that the active earth pressure is the maximum value and the passive earth pressure is the minimum value. This not only completes the mathematical proofs for Coulomb's theory of earth pressures, but also enhances the physical understanding

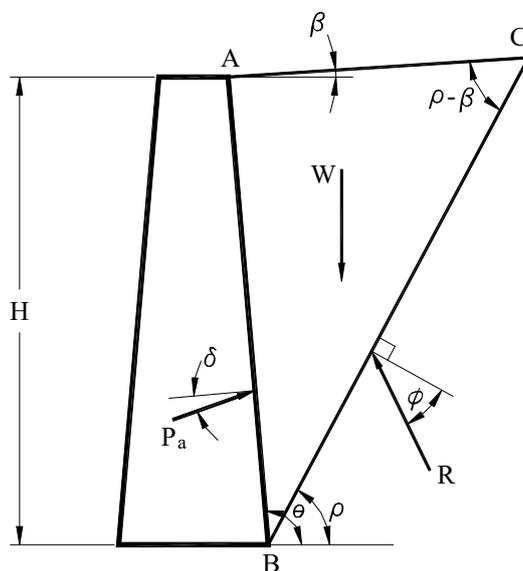
and mathematical basis of the calculation of Coulomb's earth pressures.

Keywords: Coulomb's theory, active and passive earth pressures, governing equation.

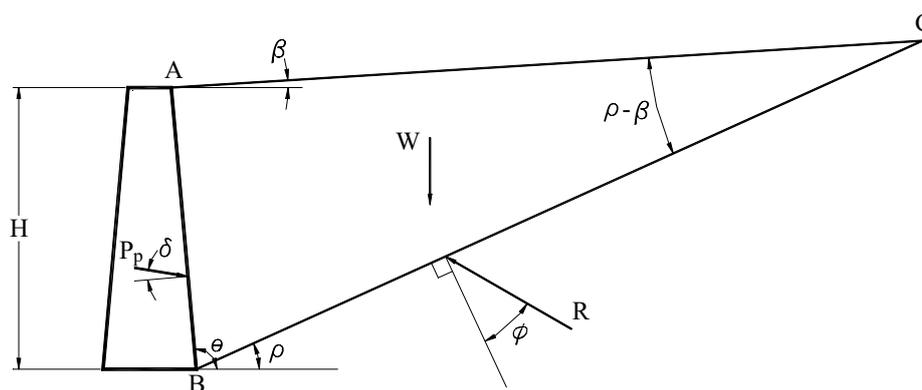
Introduction

In Figures 1(a) and (b), ΔABC is the potential active and the potential passive sliding failure block for a retaining wall, as proposed by Coulomb (1776). Here, H is the height of the retaining wall; γ is the unit weight of the soil; W is the weight per unit length of ΔABC ; β is the inclination angle of \overline{AC} ; θ is the inclination angle of \overline{AB} ; ρ is the inclination angle

of the potential active failure surface \overline{BC} ; $\rho - \beta$ is the angle between \overline{CA} and \overline{CB} ; R is the resultant shear resisting force acting on \overline{BC} , the intersection angle of R and the normal line of \overline{BC} is internal friction angle ϕ ; and P_a is the active earth pressure acting on \overline{AB} , and the angle of intersection between P_a and the normal of \overline{AB} is wall friction angle δ .



(a) Under active conditions



(b) Under passive conditions

Figure 1. Various forces on a retaining wall provided by Coulomb's potential sliding failure block.

When the inclination angles of the potential sliding failure surfaces corresponding to the active and passive earth pressure functions are changed such that P_a becomes a maximum value and P_p becomes a minimum value, P_a and P_p respectively become Coulomb's active and passive earth pressures (Coulomb, 1776).

Because Coulomb's earth pressure function (Coulomb, 1776) is too complicated, so far none of the current literatures (Bowles, 1988; Das, 2010; Fang, et al., 2003; Lambe and Whitman, 1969; McCarthy, 2007; Kramer, 1996; Pantelidis, 2019; Rebhann, 1987; Taylor, 1948;

Terzaghi, 1944; US Army Corps of Engineers, 2019) providing complete mathematical proofs of active earth pressure P_a as the maximum value and passive earth pressure P_p as the minimum value. Therefore, in this paper, the authors will directly provide Coulomb's earth pressure function first-order differential equation equal to zero, and second-order differential equation less than and greater than zero, respectively, to provide complete mathematical proofs of Coulomb's active earth pressure P_a as the maximum value and passive earth pressure P_p as the minimum value.

Coulomb's Earth Pressure Functions

In Fig. 1 and Fig. 2, the potential sliding failure blocks under Coulomb's active and passive con-

ditions (Coulomb, 1776), respectively, are given by ΔABC , where the weight W of ΔABC can be calculated as follows:

$$\begin{aligned}
 W &= \frac{1}{2} \gamma \overline{BC} \cdot \overline{AB} \cdot \sin(\theta - \rho) \\
 &= \frac{1}{2} \gamma \overline{AB}^2 \cdot \frac{\sin(180^\circ - \theta + \beta)}{\sin(\rho - \beta)} \cdot \sin(\theta - \rho) \\
 &= \frac{1}{2} \gamma H^2 \frac{\sin(180^\circ - \theta + \beta)}{\sin(\rho - \beta)} \cdot \frac{\sin(\theta - \rho)}{\sin^2(180^\circ - \theta)} \\
 &= \frac{1}{2} \gamma H^2 \frac{\sin(\theta - \beta)}{\sin^2 \theta} \cdot \frac{\sin(\theta - \rho)}{\sin(\rho - \beta)}. \tag{Equation 1}
 \end{aligned}$$

Coulomb's active earth pressure function

the conditions of force balance, the force polygon is closed.

Figure 2 shows the force polygon of W , R , and P_a in Figure 1(a). Under

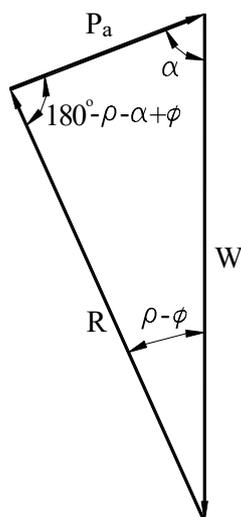


Figure 2. The closed force polygon of W , R , and P_a shown in Figure 1(a).

According to the law of sines, P_a shown in Figure 2 can be determined as follows:
 Coulomb's active earth pressure

$$P_a = W \frac{\sin(\rho - \phi)}{\sin(180^\circ - \rho - \alpha + \phi)}$$

$$= \frac{1}{2} \gamma H^2 \frac{\sin(\theta - \beta)}{\sin^2 \theta} \cdot \frac{\sin(\theta - \rho)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho - \phi)}{\sin(\rho + \alpha - \phi)}. \quad (\text{Equation 2})$$

Coulomb's active earth pressure P_a can also be expressed as:

$$P_a = \frac{1}{2} \gamma H^2 K_a \quad (\text{Equation 3})$$

where K_a is the coefficient of active earth pressure. It is defined as:

$$K_a = \frac{\sin(\theta - \beta)}{\sin^2 \theta} \cdot \frac{\sin(\theta - \rho)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho - \phi)}{\sin(\rho + \alpha - \phi)}. \quad (\text{Equation 4})$$

Coulomb's passive earth pressure function

Under the conditions of force balance, the force polygon is closed.

Figure 3 shows the force polygon of W, R, and P_p in Figure 1(b).

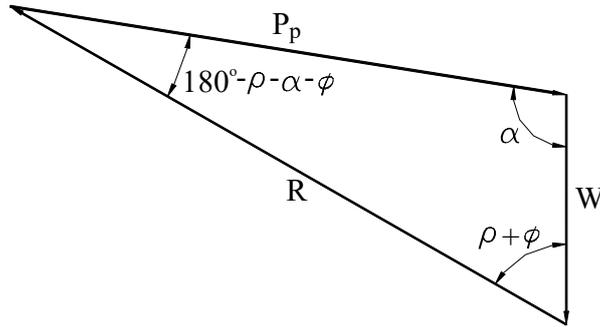


Figure 3. The closed force polygon of W, R, and P_p shown in Figure 1(b)

According to the law of sines, Coulomb's passive earth

pressure P_p shown in Figure 3 can be obtained as follows:

$$\begin{aligned}
 P_p &= W \frac{\sin(\rho + \phi)}{\sin(180^\circ - \rho - \alpha - \phi)} \\
 &= \frac{1}{2} \gamma H^2 \frac{\sin(\theta - \beta)}{\sin^2 \theta} \cdot \frac{\sin(\theta - \rho)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho + \phi)}{\sin(\rho + \alpha + \phi)}. \quad \text{(Equation 5)}
 \end{aligned}$$

Coulomb's passive earth pressure P_p can also be expressed as:

pressed as:

$$P_p = \frac{1}{2} \gamma H^2 K_p \quad \text{(Equation 6)}$$

where K_p is the coefficient of passive earth pressure. It is defined as:

defined as:

$$K_p = \frac{\sin(\theta - \beta)}{\sin^2 \theta} \cdot \frac{\sin(\theta - \rho)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho + \phi)}{\sin(\rho + \alpha + \phi)}. \quad \text{(Equation 7)}$$

Governing Equation For Coulomb's
 Earth Pressures

*Governing equation for Coulomb's ac-
 tive earth pressure*

$$\cot(\theta - \rho) + \cot(\rho - \beta) = \cot(\rho - \phi) - \cot(\rho + \alpha - \phi). \quad (\text{Equation 8})$$

*Governing equation for Coulomb's
 passive earth pressure*

For the case $\alpha = 180^\circ - \theta + \delta$, if

$$\cot(\theta - \rho) + \cot(\rho - \beta) = \cot(\rho + \phi) - \cot(\rho + \alpha + \phi). \quad (\text{Equation 9})$$

Coulomb's Theory Of Earth Pressure

*Coulomb's theory of active earth pres-
 sure*

If and only if Coulomb's active
 earth pressure function shown in Equa-
 tion 2 exists, then Equation 8 is the
 governing equation for solving Cou-
 lomb's active earth pressure P_a .

*Coulomb's theory of passive earth
 pressure*

If and only if Coulomb's passive

For the case $\alpha = 180^\circ - \theta - \delta$, if
 and only if $\rho \neq \theta$, $\rho \neq \phi$, $\rho \neq \beta$,
 and $\rho \neq \phi - \alpha$, then the governing
 equation for Coulomb's active earth
 pressure is:

and only if $\rho \neq \theta$ and $\rho \neq \beta$, then
 the governing equation for Coulomb's
 passive earth pressure is:

earth pressure function shown in Equa-
 tion 5 exists, then Equation 9 is the
 governing equation for solving Cou-
 lomb's passive earth pressure P_p .

Mathematical Proofs

*Mathematical proof of Coulomb's ac-
 tive earth pressure*

1) If H , γ , θ , β , ϕ , and δ are
 known, then $\frac{1}{2} \gamma H^2 \frac{\sin(\theta - \beta)}{\sin^2 \theta}$ in

Equation 2 is a constant C_a , and
 $\frac{\sin(\theta - \rho)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho - \phi)}{\sin(\rho + \alpha - \phi)}$ only

changes with the change of ρ .

sure function can be rewritten as:

Thus, Coulomb's active earth pres-

$$P_a = -C_a \cdot f_a(\rho).$$

In the above formula, since

$$-f_a(\rho) = \frac{\sin(\rho - \theta)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho - \phi)}{\sin(\rho + \alpha - \phi)},$$

the extreme value of Coulomb's active earth pressure can be obtained by using

the formula $\frac{dP_a}{d\rho} = C_a \cdot \frac{d[-f_a(\rho)]}{d\rho} = 0$.

Then, the governing equation for solving Coulomb's active earth pressure P_a can be obtained as follows:

$$\begin{aligned} \frac{d[-f_a(\rho)]}{d\rho} &= \frac{\cos(\rho - \theta)}{\sin(\rho - \theta)} [-f_a(\rho)] + \frac{\cos(\rho - \phi)}{\sin(\rho - \phi)} [-f_a(\rho)] \\ &\quad - \frac{\cos(\rho - \beta)}{\sin(\rho - \beta)} [-f_a(\rho)] - \frac{\cos(\rho + \alpha - \phi)}{\sin(\rho + \alpha - \phi)} [-f_a(\rho)] \\ &= [\cot(\rho - \theta) + \cot(\rho - \phi) - \cot(\rho - \beta) - \cot(\rho + \alpha - \phi)] \cdot [-f_a(\rho)] \\ &= 0. \end{aligned}$$

For general retaining walls, since

$\rho \neq \theta$, $\rho \neq \phi$, $\rho \neq \beta$, $\rho \neq \phi - \alpha$,
 and $-f_a(\rho) > 0$ in the above formula,

the governing equation of $\frac{dP_a}{d\rho} = 0$ is

obtained as

$$\begin{aligned} &\cot(\theta - \rho) + \cot(\rho - \beta) \\ &= \cot(\rho - \phi) - \cot(\rho + \alpha - \phi). \end{aligned}$$

When $\cot(\theta - \rho) + \cot(\rho - \beta) = \cot(\rho - \phi) - \cot(\rho + \alpha - \phi)$ is known, it is necessary to further prove that Coulomb's active earth pressure is the maximum value.

Since both C_a and $-f_a(\rho)$ are greater than zero, to prove that Coulomb's active earth pressure P_a is a maximum value, it is necessary to either

prove $\frac{d^2 P_a}{d\rho^2} = C_a \cdot \frac{d^2 [-f_a(\rho)]}{d\rho^2} < 0$ or

prove $\frac{d^2 [-f_a(\rho)]}{d\rho^2} < 0$.

For general retaining walls, since $45^\circ < \theta < 135^\circ$, $|\beta| \leq \phi$, and $\delta \leq \phi$, the value of ρ that satisfies Equation 8 also satisfies $|\rho - \theta| < |\rho + \alpha - \phi|$ and

$|\rho - \phi| < |\rho - \beta|$. Therefore,
 $\csc^2(\rho + \alpha - \phi) < \csc^2(\rho - \theta)$ and
 $\csc^2(\rho - \beta) < \csc^2(\rho - \phi)$ both exist.

Under such circumstances, the second-order differential equation of $-f_a(\rho)$ can be obtained as follows:

$$\frac{d^2[-f_a(\rho)]}{d\rho^2} = [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha - \phi) - \csc^2(\rho - \phi) + \csc^2(\rho - \beta)] \cdot [-f_a(\rho)] \\ + [\cot(\rho - \theta) - \cot(\rho + \alpha - \phi) + \cot(\rho - \phi) - \cot(\rho - \beta)] \cdot \frac{d[-f_a(\rho)]}{d\rho}.$$

Since
 $\cot(\theta - \rho) + \cot(\rho - \beta) =$

$\cot(\rho - \phi) - \cot(\rho + \alpha - \phi)$, the above formula can be simplified to:

$$\frac{d^2[-f_a(\rho)]}{d\rho^2} = [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha - \phi) - \csc^2(\rho - \phi) + \csc^2(\rho - \beta)] \cdot [-f_a(\rho)] \\ < 0.$$

This completes the mathematical proof of the *if* component "if Coulomb's active earth pressure function P_a shown in Equation 2 exists, then Equation 8 is the governing equation for solving Coulomb's active earth pressure P_a ."

2) Since Equation 8 exists, the value of ρ that satisfies Equation 8 also makes $\sin(\rho - \theta)$, $\sin(\rho - \phi)$, $\sin(\rho - \beta)$, and $\sin(\rho + \alpha - \phi)$ non-zero. Therefore, after multiplying both sides of Equation 8 by $-f_a(\rho)$, the following formula can be obtained:

$$[\cot(\rho - \theta) + \cot(\rho - \phi) - \cot(\rho - \beta) - \cot(\rho + \alpha - \phi)] \cdot [-f_a(\rho)] \\ = \frac{\cos(\rho - \theta)}{\sin(\rho - \theta)} [-f_a(\rho)] + \frac{\cos(\rho - \phi)}{\sin(\rho - \phi)} [-f_a(\rho)] \\ - \frac{\cos(\rho - \beta)}{\sin(\rho - \beta)} [-f_a(\rho)] - \frac{\cos(\rho + \alpha - \phi)}{\sin(\rho + \alpha - \phi)} [-f_a(\rho)] \\ = \frac{d[-f_a(\rho)]}{d\rho} \\ = 0.$$

For general retaining walls, since $45^\circ < \theta < 135^\circ$, $|\beta| \leq \phi$, and $\delta \leq \phi$, the constant C_a and $-f_a(\rho)$ are both greater than zero. Therefore, the value of ρ that satisfies Equation 8

also makes $|\rho - \theta| < |\rho + \alpha - \phi|$ and $|\rho - \phi| < |\rho - \beta|$. Therefore, $\csc^2(\rho - \theta) > \csc^2(\rho + \alpha - \phi)$ and $\csc^2(\rho - \phi) > \csc^2(\rho - \beta)$. Thus, the following relationship exists:

$$\begin{aligned} & \left[-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha - \phi) - \csc^2(\rho - \phi) + \csc^2(\rho - \beta) \right] \cdot [-f_a(\rho)] \\ &= \left[-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha - \phi) - \csc^2(\rho - \phi) + \csc^2(\rho - \beta) \right] \cdot [-f_a(\rho)] \\ & \quad + \left[\cot(\rho - \theta) - \cot(\rho + \alpha - \phi) + \cot(\rho - \phi) - \cot(\rho - \beta) \right] \cdot \frac{d[-f_a(\rho)]}{d\rho} \\ &= \frac{d^2[-f_a(\rho)]}{d\rho^2} < 0. \end{aligned}$$

This completes the mathematical proof of the *only if* component: "only if the governing equation of Coulomb's active earth pressure P_a shown in Equation 8 exists, then P_a shown in Equation 2 is Coulomb's active earth pressure."

Mathematical proof of Coulomb's passive earth pressure

1) If H , γ , θ , β , ϕ , and δ are

known, then $\frac{1}{2} \gamma H^2 \frac{\sin(\theta - \beta)}{\sin^2 \theta}$ in

Equation 5 is a constant C_p , and $\frac{\sin(\rho - \theta)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho + \phi)}{\sin(\rho + \alpha + \phi)}$ only changes with the change of ρ . Thus, Coulomb's passive earth pressure function P_p can be rewritten as:

$$P_p = -C_p \cdot f_p(\rho)$$

In the above formula, since

$$-f_p(\rho) = \frac{\sin(\rho - \theta)}{\sin(\rho - \beta)} \cdot \frac{\sin(\rho + \phi)}{\sin(\rho + \alpha + \phi)},$$
the extreme value of Coulomb's passive earth pressure can be obtained by using the formula

$$\frac{dP_p}{d\rho} = C_p \cdot \frac{d[-f_p(\rho)]}{d\rho} = 0.$$
 Then, the governing equation for solving Coulomb's passive earth pressure P_p can be obtained as follows:

$$\begin{aligned} \frac{d[-f_p(\rho)]}{d\rho} &= \frac{\cos(\rho - \theta)}{\sin(\rho - \theta)} [-f_p(\rho)] + \frac{\cos(\rho + \phi)}{\sin(\rho + \phi)} [-f_p(\rho)] \\ &\quad - \frac{\cos(\rho - \beta)}{\sin(\rho - \beta)} [-f_p(\rho)] - \frac{\cos(\rho + \alpha + \phi)}{\sin(\rho + \alpha + \phi)} [-f_p(\rho)] \\ &= [\cot(\rho - \theta) + \cot(\rho + \phi) - \cot(\rho - \beta) - \cot(\rho + \alpha + \phi)] \cdot [-f_p(\rho)] \\ &= 0. \end{aligned}$$

For general retaining walls, since $\rho \neq \theta$ and $\rho \neq \beta$, $\sin(\rho - \theta)$, $\sin(\rho + \phi)$, $\sin(\rho - \beta)$, and $\sin(\rho + \alpha + \phi)$ are all non-zero, and $-f_p(\rho) > 0$ in the above formula, the governing equation for $dP_p/d\rho = 0$ is $\cot(\theta - \rho) + \cot(\rho - \beta) = \cot(\rho + \phi) - \cot(\rho + \alpha + \phi)$.

When $\cot(\theta - \rho) + \cot(\rho - \beta) = \cot(\rho + \phi) - \cot(\rho + \alpha + \phi)$ is known, it is necessary to further prove that Coulomb's passive earth pressure is the minimum value.

Since both C_p and $-f_p(\rho)$ are

greater than zero, to prove that P_p is a minimum value, it is necessary to prove either $\frac{d^2P_p}{d\rho^2} = C_p \cdot \frac{d^2[-f_p(\rho)]}{d\rho^2} > 0$ or $\frac{d^2[-f_p(\rho)]}{d\rho^2} > 0$.

For general retaining walls, since $45^\circ < \theta < 135^\circ$, $|\beta| \leq \phi$, and $\delta \leq \phi$, the value of ρ that satisfies Equation 9 also satisfies $|\rho - \theta| > |180^\circ - \rho - \alpha - \phi|$ and $|\rho + \phi| > |\rho - \beta|$. Therefore, $[-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha + \phi)] > 0$ and $[-\csc^2(\rho + \phi) + \csc^2(\rho - \beta)] > 0$ both exist. Under such circumstances, the second-order differential formula of $-f_p(\rho)$ can be obtained as follows:

$$\frac{d^2[-f_p(\rho)]}{d\rho^2} = [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha + \phi) - \csc^2(\rho + \phi) + \csc^2(\rho - \beta)] \cdot [-f_p(\rho)] + [\cot(\rho - \theta) - \cot(\rho + \alpha + \phi) + \cot(\rho + \phi) - \cot(\rho - \beta)] \cdot \frac{d[-f_p(\rho)]}{d\rho}.$$

In the above formula, since $\cot(\theta - \rho)$ the following is obtained:

$$+\cot(\rho - \beta) = \cot(\rho + \phi) - \cot(\rho + \alpha + \phi),$$

$$\frac{d^2[-f_p(\rho)]}{d\rho^2} = [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha + \phi) - \csc^2(\rho + \phi) + \csc^2(\rho - \beta)] \cdot [-f_p(\rho)] > 0.$$

This completes the mathematical proof of the *if* component: "If Coulomb's passive earth pressure function P_p shown in Equation 5 exists, then Equation 9 is the governing equation for solving Coulomb's passive earth pressure."

When Equation 9 exists, the value of ρ that satisfies Equation 9 also

makes $\sin(\rho - \theta)$, $\sin(\rho + \phi)$, $\sin(\rho - \beta)$, and $\sin(\rho + \alpha + \phi)$ non-zero, and because $\sin(\rho + \phi) > 0$ and $\sin(\rho + \alpha + \phi) > 0$, the following formula can be obtained by multiplying both sides of

$$\begin{aligned} &\cot(\rho - \theta) - \cot(\rho + \alpha + \phi) \\ &+ \cot(\rho + \phi) - \cot(\rho - \beta) = 0 \text{ by} \\ &-f_p(\rho). \end{aligned}$$

$$\begin{aligned} &\cot(\rho - \theta)[-f_p(\rho)] + \cot(\rho + \phi)[-f_p(\rho)] \\ &- \cot(\rho - \beta)[-f_p(\rho)] - \cot(\rho + \alpha + \phi)[-f_p(\rho)] \\ &= \frac{\cos(\rho - \theta)}{\sin(\rho - \theta)}[-f_p(\rho)] + \frac{\cos(\rho + \phi)}{\sin(\rho + \phi)}[-f_p(\rho)] \\ &- \frac{\cos(\rho - \beta)}{\sin(\rho - \beta)}[-f_p(\rho)] - \frac{\cos(\rho + \alpha + \phi)}{\sin(\rho + \alpha + \phi)}[-f_p(\rho)] \\ &= \frac{d[-f_p(\rho)]}{d\rho} \\ &= 0. \end{aligned}$$

For general retaining walls, since $45^\circ < \theta < 135^\circ$, $|\beta| \leq \phi$, and $\delta \leq \phi$, the constant C_a and $-f_a(\rho)$ are both greater than zero. Thus, ρ satisfying Equation 9 also leads to

$$C_p \cdot \frac{d[-f_p(\rho)]}{d\rho} = \frac{dP_p}{d\rho} = 0. \text{ Furthermore,}$$

$$\begin{aligned} \frac{d^2[-f_p(\rho)]}{d\rho^2} &= [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha + \phi) - \csc^2(\rho + \phi) + \csc^2(\rho - \beta)] \cdot [-f_p(\rho)] \\ &\quad + [\cot(\rho - \theta) - \cot(\rho + \alpha + \phi) + \cot(\rho + \phi) - \cot(\rho - \beta)] \cdot \frac{d[-f_p(\rho)]}{d\rho} \\ &= [-\csc^2(\rho - \theta) + \csc^2(\rho + \alpha + \phi) - \csc^2(\rho + \phi) + \csc^2(\rho - \beta)] \cdot [-f_p(\rho)] \\ &> 0. \end{aligned}$$

This completes the mathematical proof of the *only if* component: “Only if the governing equation of Coulomb’s active earth pressure P_p shown in Equation 9 exists, then P_p shown in Equation 5 is Coulomb’s passive earth pressure.”

Comparison And Discussion Of Results

1) Rebhann (1871) first obtained the relationship between Coulomb’s active earth pressure P_a and the weight W for the potential sliding failure block by us-

because $|\rho - \theta| > |180^\circ - \rho - \alpha - \phi|$ and $|\rho + \phi| > |\rho - \beta|$ exist, it is known that $\csc^2(\rho - \theta) < \csc^2(\rho + \alpha + \phi)$ and $\csc^2(\rho + \phi) < \csc^2(\rho - \beta)$. Therefore, the following relationship can be obtained:

ing the closed force polygon of W , R , and P_a shown in Figure 3 and the law of sines. A drawing method (detailed in Figure 5) is used to illustrate that when the inclination angle ρ of the sliding failure surface changes such that $dP_a/d\rho = 0$, then the area of ΔABC equals the area of ΔBCE . Finally, Rebhann derived a formula for the coefficient of lateral earth pressure K_a under active conditions:

$$K_a = \frac{\sin^2(\theta - \phi)}{\sin^2 \theta \cdot \sin(\theta + \delta) \left(1 + \sqrt{\frac{\sin(\phi + \delta) \cdot \sin(\phi - \beta)}{\sin(\theta + \delta) \cdot \sin(\theta - \beta)}} \right)^2} \quad \text{(Equation 10)}$$

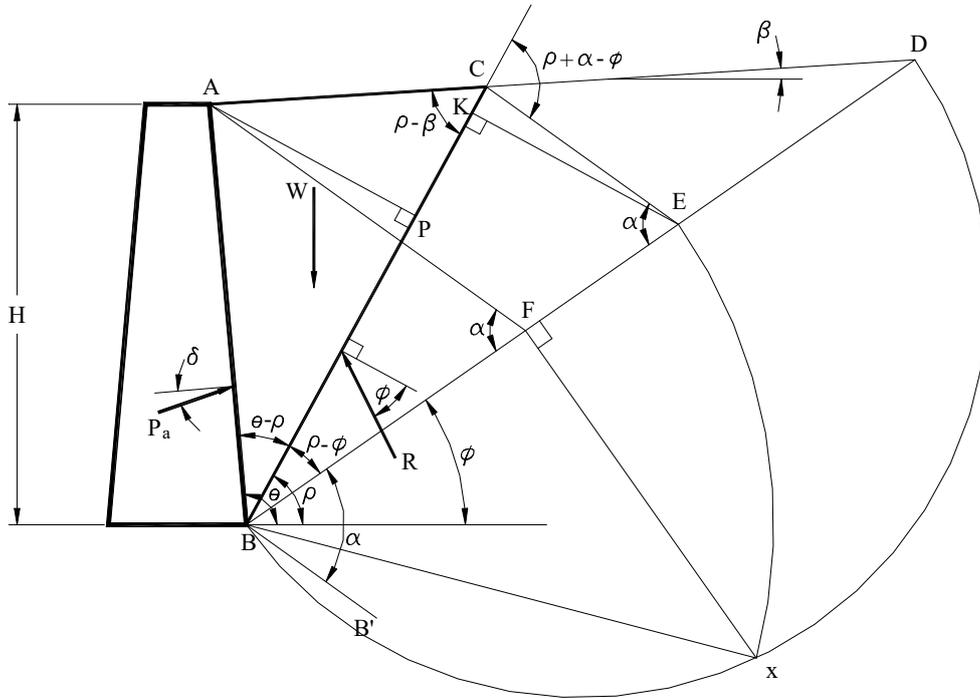


Figure 5. Rebhann's drawing method for Coulomb's active earth pressure.

2) Rebhann (1871) also obtained the relationship between Coulomb's passive earth pressure function P_p and the weight W of the potential sliding failure block by using the closed force polygon of the three forces of W , R , and P_p shown in Figure 4 and the law of sines. Then, a drawing method (detailed in Figure 6) was used to illustrate

that when the inclination angle ρ of the sliding failure surface is changed such that $dP_a/d\rho = 0$, then the area of $\triangle ABC$ equals the area of $\triangle BCE$. Finally, Rebhann derived a formula for the coefficient of lateral earth pressure K_a under passive conditions:

$$K_p = \frac{\sin^2(\theta + \phi)}{\sin^2 \theta \cdot \sin(\theta - \delta) \left(1 - \sqrt{\frac{\sin(\phi + \delta) \cdot \sin(\phi + \beta)}{\sin(\theta - \delta) \cdot \sin(\theta - \beta)}} \right)^2} \quad (\text{Equation 11})$$

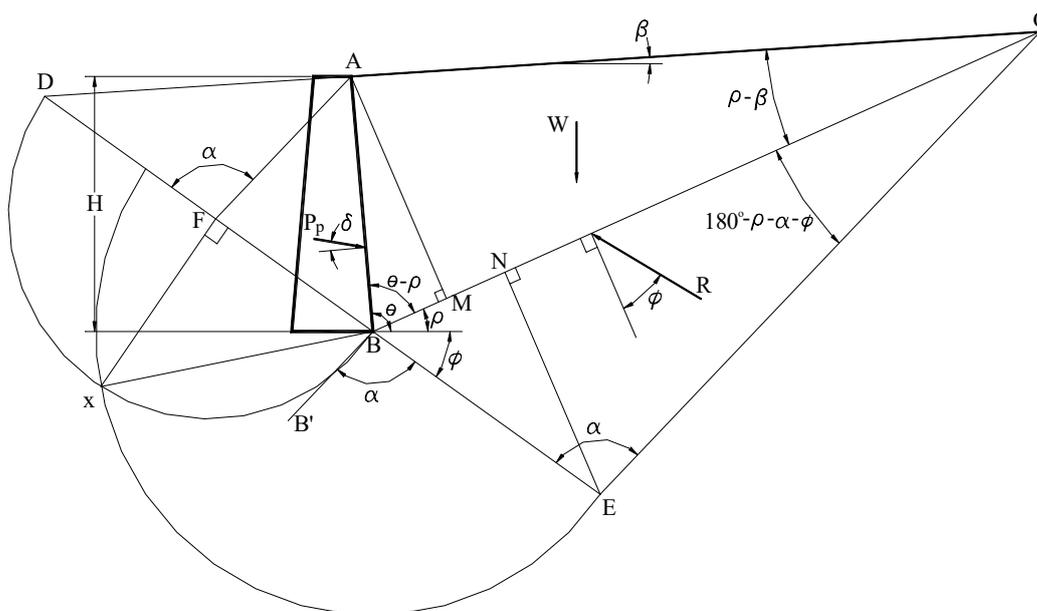


Figure 6. Rebhann's drawing method for Coulomb's passive earth pressure.

3) After obtaining the graphical relationships for $dP_a/d\rho=0$ and $dP_p/d\rho=0$ by the drawing method, Rebhann (1871) did not go on to prove that P_a is a maximum value by $d^2P_a/d\rho^2 < 0$ and that P_p is a minimum value by $d^2P_p/d\rho^2 > 0$. Therefore,

the proof for the theory of Coulomb's earth pressure is incomplete.

4) When the inclination angles of the sliding failure surfaces corresponding to the maximum value of P_a and the minimum value of P_p are required, it is easy to solve them by using Equation 8 and Equation 9,

respectively. However, it is very time-consuming and laborious work to obtain them using Rebhann's drawing methods.

$\cot(\theta - \rho) = \overline{BP} / \overline{AP}$,
 $\cot(\rho - \beta) = \overline{CP} / \overline{AP}$,
 $\cot(\rho - \phi) = \overline{BK} / \overline{EK}$, and
 $\cot(\rho + \alpha - \phi) = -\overline{CK} / \overline{EK}$ can be obtained. If the above relational expressions are substituted into Eq. 8, then it can be found that:

5) For the governing equation of Coulomb's active earth pressure, according to Figure 5,

$$\frac{\overline{BP}}{\overline{AP}} + \frac{\overline{CP}}{\overline{AP}} = \frac{\overline{BK}}{\overline{EK}} + \frac{\overline{CK}}{\overline{EK}}$$

The above formula can be simplified to:

$$\frac{\overline{BC}}{\overline{AP}} = \frac{\overline{BC}}{\overline{EK}}$$

Multiplying both sides by

$\frac{1}{2} \overline{AP} \cdot \overline{EK}$ gives:

$$\frac{1}{2} \overline{BC} \cdot \overline{AP} = \frac{1}{2} \overline{BC} \cdot \overline{EK}$$

In Figure 5, this relationship means that the area of ΔABC equals the area of ΔBCE . Therefore, it is proved that the governing equation of Coulomb's active earth pressure proposed in this paper has the same physical meaning as the graphical relationship suitable

for $dP_a / d\rho = 0$ proposed by Rebhann (1871).

6) According to Figure 6, the same method can be used to prove that the physical meaning of the governing equation of Coulomb's passive earth pressure proposed in this paper is the

same as that of the graphical relationship suit

able for $dP_p/d\rho = 0$ proposed by Rebhann (1871).

7) Take the retaining wall shown in Figures 1 or 2 as examples. When $H=6$ m, $\gamma=20$ kN/m³, $\theta=120^\circ$, $\beta=12^\circ$, $c=0$, $\phi=30^\circ$, and $\delta=20^\circ$, the following results can be obtained according to the traditional method and the method proposed in this paper:

(1) According to the traditional method, Coulomb's active and passive lateral earth pressure coefficients, *i.e.*, $K_a=0.7882$ and $K_p=5.0000$, are first obtained using Equations 10 and 11, respectively. Then, Coulomb's active and passive earth pressures, *i.e.*, $P_a=283.752$ kN per meter of wall and $P_p=1800.000$ kN per meter of wall, are determined using Equations 3 and 6, respectively. Regarding the inclination angles ρ of the sliding failure surfaces and the weight W of the sliding failure blocks for Coulomb's active and passive conditions, technicians

rarely present these data because they require significant drawing efforts.

(2) According to the method proposed in this paper, by using the governing equations of Equations 8 and 9, the inclination angle (*i.e.*, $\rho = 57.57^\circ$) of the sliding failure surface for Coulomb's active conditions and the inclination angle (*i.e.*, $\rho = 40.00^\circ$) of the sliding failure surface for Coulomb's passive conditions can be obtained. Then, the known values of ρ can be substituted into Equation 1 to obtain the weight (*i.e.*, $W = 566.679$ kN per meter of wall) for the sliding failure block of Coulomb's active conditions and the weight (*i.e.*, $W = 969.786$ kN per meter of wall) for the sliding failure block of Coulomb's passive conditions. When the known values of ρ are substituted into Equations 2 and 5, this yields $P_a = 283.752$ kN per meter of wall and $P_p = 1800.000$ kN per meter of wall for the Coulomb's active and passive conditions,

respectively. Finally, when the known values of ρ are substituted into Equations 4 and 7, the Coulomb active earth pressure coefficient (*i.e.*, $K_a=0.7882$) and the Coulomb passive earth pressure coefficient (*i.e.*, $K_p=5.0000$) can be obtained.

Conclusions And Suggestions

Coulomb's theory of earth pressure has been widely used for 245 years in the design of retaining walls. However, because the earth pressure functions are very complicated, there have been no complete mathematical proofs. To complete Coulomb's theory of earth pressure, complete mathematical proofs were provided for the first time in this paper.

Through comparison, it is known that the proposed governing equations can be easily used to calculate the inclination angle ρ of the sliding failure surface, the weight W of the sliding block, the active earth pressure P_a , the passive earth pressure P_p , the active earth pressure coefficient K_a , and the passive earth pressure coefficient K_p for Coulomb's active and passive earth pressure conditions.

Then, governing equations for Coulomb's active and passive earth pressures were proved to have the same physical meanings as the graphical relationships obtained by satisfying $dP_a/d\rho=0$ and $dP_p/d\rho=0$ in Rebhann's drawing methods.

Based on the above research conclusions, the authors strongly suggest that when designing retaining walls in the future, for the convenience of calculation, the governing equations proposed in this paper for the Coulomb's active and passive earth pressures should be used directly. It is also suggested to incorporate the complete mathematical proofs provided in this paper into foundation engineering design textbooks and design specifications to enable the Coulomb's theory of active and passive earth pressures to be complete.

References

- Bowles, J. E., *Foundation Analysis and Design*, 4th Edition, McGraw-Hill Book Company, pp. 476-483, 1988.
- Coulomb, C. A., "Essai sur une application des regles des maximis et minimis a quelques problemes de statique relatifs a l'Architecture,"

- Memoires de Mathematique et de Physique, Presentes a l'Academie Royale des Sciences, par divers Savans, et lus dans ses Assemblies, Paris, Vol. 7, (volume for 1773), pp. 343-382, 1776.
- Das, B. M., *Principles of Foundation Engineering*, 7th Edition, PWS Publishing Company, Boston, pp. 273-308, 2010.
- Fang, Y.S., Yang, Y.C. and Chen, T. J., "Retaining walls damaged in the Chi-Chi earthquake," *Canadian Geotechnical Journal*, Vol. 40, pp. 1142-1153, Website: <https://ir.nctu.edu.tw/bitstream/11536/27350/1/000187035900006.pdf>, 2003.
- Lambe, T. W. and Whitman, R. V., *Soil Mechanics*, John Wiley & Sons, New York, pp. 178-179, 1969.
- McCarthy, D. F., *Essentials of Soil Mechanics and Foundations*, 7th Edition, New Jersey, Prentice Hall, p. 730-733, 2007.
- Kramer, S. L., *Geotechnical Earthquake Engineering*, Prentice Hall, New Jersey, pp. 472-474 and pp.478-481, 1996.
- Pantelidis, L., "The Generalized Coefficients of Earth Pressure: A Unified Approach," *Applied Sciences*, pp. 1-42, Website: <file:///D:/C-DATA/Downloads/applsci-09-05291-v3.pdf>, 4 December 2019.
- Rebhann, G., *Theorie des Erd-druckes und der Futtermauern*, Vienna, 1871.
- Taylor, D. W., *Fundamentals of Soil Mechanics*, John Wiley and Sons, New York, pp. 480-527, 1948.
- Terzaghi, Karl, *Theoretical Soil Mechanics*, John Wiley and Sons, New York, pp. 71-117, 1943.
- US Army Corps of Engineers, "Seismic Retaining Wall Failure," *Best Practices in Dam and Levee Safety Risk Analysis Part E-Concrete Structures*, Chapter E-7, Website: <https://www.usbr.gov/ssle/damsafety/risk/BestPractices/Presentations/E7-SeismicRetainingWallFailurePP.pdf>, 2019.